Trahan
ARCHITECTURE & PLANNING, LLC

February 9, 2026

ADDENDUM NO. 2

Project:

A new racket center & observation deck for:
ST JULIEN RECREATIONAL FACILITY
701 St. Nazaire Road
Broussard, LA 70518

To all plan Holders:
This addendum forms a part of the bidding and contract documents and modifies the original contract documents dated 1-5-26. Bidder must acknowledge receipt of ALL addendums in the space provided on the bid form. Failure to acknowledge receipt of addendum shall be cause for rejection of bid.

Failure to do so shall subject the bidder to disqualification.

Items:

1. General:
    a. Builders Risk Insurance:
        i. The Contractor shall effect and maintain until substantial completion of the work, insurance for Builders Risk for projects involving building construction.
        ii. The Owner assumes no risk for loss by fire or other casualty to any portion of the construction project or equipment thereof, whether completed, in process of construction or installation, or stored on the premises, during the life of any contract for any portion of the construction.
        iii. When required by the Owner, the Contractor shall maintain during the life of the Contract insurance acceptable to the Owner against all risks. This insurance shall be an amount equal to the amount of the Contract. Such policy shall include the Owner as a named insured and shall be furnished to the Owner through its Risk Management Division prior to execution of the contract. Contractor shall be responsible for any and all deductibles.
        iv. The making of partial payments to the Contractor shall not be construed as creation of an insurable interest by or for the Owner or as relieving the various contractors or their sureties of responsibility from loss from all

819 Saint John St.
Lafayette, LA 70501
337.504-7580

risks (fire, windstorm, explosion, vandalism, flood, etc.) occurring prior to substantial completion of the project.

b. Hydroseeding:
    i. Upon completion of construction, the contractor shall furnish and install hydroseeding on all areas within and adjacent to the construction limits that were disturbed by construction activities.
c. Geotechnical Report:
    i. The attached Geotechnical Report shall be added to the project documents.

2. Specifications:
    a. Instructions to Bidders
        i. Liquidated Damages:
            1. Liquidated Damages shall be <u>One Thousand Five Hundred dollars</u> ($1,500.00) for each consecutive calendar day.
    b. Section 051200-Structural Steel Framing
        i. 1.5, Quality Assurance
            1. Fabricator Qualifications:
                a. Where a shop does not participate in the AISC quality certification program, the shop shall submit comply and submit the following for review by the engineer:
                    i. Documentation indicating Fabricator with a minimum of Five (5) years of experience on similar scale projects.
                    ii. Submit list of representative (similar scale) projects.
                    iii. Shop QC procedures.
                    iv. Welder certifications.

3. Drawings:
    a. Sheet A4.4:
        i. Detail 9/A4.4:
            1. Stainless Steel counter-top and brackets shall be 14ga S.S..
    b. Sheet S2.2, Rev-1 (2/9/2026) attached.
        i. The attached revised sheet S2.2, Rev-1, shall replace the original in it's entirety.
    c. Sheet S4.0, Rev-1 (2/9/2026) attached.

i. The attached revised sheet S4.0, Rev-1, shall replace the original in it's entirety.

End of Addendum

819 Saint John St.
Lafayette, LA 70501
337.504-7580

GEOTECHNICAL INVESTIGATION

FOR

ST. JULIEN PARK PICKLEBALL COURTS
ST. NAZAIRE ROAD
BROUSSARD, LA

LTI Project No. 4603-BO

PREPARED FOR
CITY OF BROUSSARD
C/O COMEAUX ENGINEERING
BROUSSARD, LA

March 10, 2025

PREPARED BY:

LOUISIANA TESTING & INSPECTION
LAFAYETTE, BATON ROUGE, HOUMA

P.O. BOX 2934
LAFAYETTE, LOUISIANA 70502
EMAIL: LOUISIANATESTING@GMAIL.COM
PHONE: 337-235-9411
FAX: 337-232-9362

Louisiana Testing & Inspection, Inc.

P. O. Box 2934
Lafayette, Louisiana 70502

Telephone 337-235-9411
Fax 337-232-9362
Email - Louisianatesting@gmail.com

March 10, 2025

City of Broussard
C/O Comeaux Engineering
P.O. Box 452
Broussard, LA 70518

Attention: Mr. Daniel Hutchinson P.E.
E-mail: daniel@comeauxengineering.com

Re:
Geotechnical Investigation Report
St. Julien Park Pickleball Courts
St. Nazaire Road
Broussard, LA
LTI File No. 4603-BO

Dear Daniel,

Please find attached our geotechnical investigation report that was completed for the referenced project. We appreciate the opportunity to serve your geotechnical needs. Please contact us should you have any questions.

Sincerely,
LOUISIANA TESTING AND INSPECTION, INC

CHAD M. POCHE, P.E.

Copies Submitted: 2 (1 bound and 1 unbound)

TABLE OF CONTENTS

Louisiana Testing & Inspection, Inc.

GEOTECHNICAL INVESTIGATION REPORT

ST. JULIEN PARK PICKLEBALL COURTS
ST. NAZAIRE ROAD
BROUSSARD, LA

LOUISIANA TESTING AND INSPECTION (LTI) FILE NO. 4603-BO

INTRODUCTION

This report contains the results of a geotechnical investigation made at the referenced site. Instructions to proceed with the investigation were received from City of Broussard C/O Comeaux Engineering (Client).

The study included the drilling of soil test borings and the performance of soil mechanics laboratory tests to evaluate the soil's physical characteristics. Analyses were made and based on the field and laboratory test data to develop recommendations for the project.

The analyses and recommendations presented in this report are based on the project information provided and the results of the investigation. While it is not likely that conditions will differ significantly from those observed during the field investigation it is always possible that variations can occur away from the borehole locations.

If it becomes apparent during construction that subsurface conditions differing significantly from those observed in our borings are being encountered, LTI should be notified at once. Also, should the nature of the project change or should any of the stated assumptions be inaccurate, the recommendations provided in this report should be re-evaluated.

This report has been prepared for the exclusive use of our Client. The recommendations provided in this report are site specific and are not intended for use at any other site or for any other project. This report provides recommendations for design and construction and should not be used as construction specifications.

SOIL BORINGS

Eight (8) undisturbed sample type soil borings were each completed at a depth of 24 feet (B-1 through B-8) below the existing ground surface in the general area of the proposed project on February 3, 2025. The borings were made using a trailer mounted drill rig and backfilled upon completion in accordance with appropriate State regulations. The approximate boring locations at the site are shown on Figure 1.

Undisturbed sampling was performed continuously or on approximate 5-foot centers in all cohesive or semi-cohesive materials using a small diameter thin wall tube sampler. The samples were extruded in the field and representative portions of each sample were trimmed and placed in moisture proof containers. The samples were then properly labeled and secured for transport to the laboratory.

When cohesionless soils were encountered, disturbed samples were taken during the Standard Penetration Test. This test is performed by driving a two (2) inch O.D. split-spoon sampler one (1) foot, after first sending it six (6) inches, using blows of a 140-pound weight dropped thirty (30) inches. The results of this test give indications of in-situ characteristics of the cohesionless soils. The values obtained from this test are shown in the boring log and are a measure of the blow counts for the final 12 inches of driving.

LABORATORY TESTING

Soil mechanics laboratory tests were performed on samples obtained from the borings. The testing consisted of natural moisture content, unit weight, Atterberg limits, and unconfined compression (strength testing). The results of the laboratory tests are shown on the soil boring logs provided in the Appendix of this report.

SUBSOIL CONDITIONS

Subsoil Description

Reference to the boring logs shows there are interbedded layers of very soft to hard silty clay from the ground surface to the borings' termination depth of 24 feet.

Groundwater

At the time of making the borings, free water or groundwater was not encountered. Groundwater can fluctuate with seasonal precipitation, drainage, and prolonged drought. If the depth to groundwater is important to construction, it should be measured at that time.

FURNISHED INFORMATION AND FOUNDATION RECOMMENDATIONS

Furnished information indicates a new pickleball facility (St. Julien Park) will be constructed at St. Nazaire Road in Broussard, LA. The facility will consist of multiple pickleball courts, a pre-engineered metal building, and a two-story building that will house a pro shop. No structural loading information was provided and we assume no more than 2 feet of fill will be placed to raise the site grade.

The near surface soils within the borings are borderline with respect to supporting heavily loaded structures on shallow footings due to the presence of soft soil layers within the upper 10 feet of the ground surface. The use of shallow footings for support of lightly loaded structures appears reasonable. Alternatively, heavily loaded structures may be supported on deep foundations (cast in place drilled shafts). Any fill should be placed as far in advance of construction as possible. Analyses were made and based on the borings and laboratory test data to develop geotechnical related parameters for use in design of the structure's foundation.

For shallow foundations, precautions should be made to assure the footing excavations are well drained during construction and that good rigidity in the foundation is designed to minimize movements due to settlements. Care should be taken during and after construction to limit activities that could affect moisture within the soils below and around the foundations.

The near surface soils encountered in the borings appear to possess minimal swell potential. Swell testing was not performed. By precluding surface waters from saturating the soils, the resulting volumetric movements will be minimized. In this regard, good roof and surface drainage should be assured with positive collection and runoff of these waters away from foundations. Structural analyses and the adequacy of foundation designs are outside our scope of work for the project.

SHALLOW FOUNDATIONS

Allowable Soil Bearing Capacities

We estimate an allowable soil bearing capacity of 1,200 lbs. per sq. ft. (psf) is available for design of continuous wall footings up to 3 ft. in width, and an allowable soil bearing capacity of 1,500 psf is available for square spread footings up to 6 ft. in width.

These allowable soil bearing capacities assume the footings are seated in firm soils at a minimum depth of 2 feet below the finished ground surface. Prior to construction, the foundation areas should be stripped of all vegetation, debris, soft or loose surface soils, deleterious materials, etc., and should be well drained.

Foundation excavations should be thoroughly inspected to assure that the footings are seated in firm and well-drained soil. The allowable soil bearing capacities contain a factor of safety of at least 3.0 against failure but do not preclude settlements, as will be discussed.

Estimated Settlement - Footings

Settlement of maximum 6 ft. wide square footings and 3 ft. wide continuous footings were analyzed. The results of our analyses are provided on the following table. Settlement will increase with the size of the spread footings and if larger footings are needed for support additional settlement analyses should be made.

NET APPLIED BEARING PRESSURE (PSF)	ESTIMATED SETTLEMENT (INCHES)
900	Up to ½
1,200	½ to ¾
1,500	¾ to 1

Adequate steel reinforcement should be designed and included within the foundation. If the estimated settlements for shallow footings or slabs are considered prohibitive, deep foundations should be used to support the structure.

Estimated Settlement – Slab

Analyses were made to estimate long term consolidation settlement at the center of an approximate 50 ft. by 50 ft. slab. We estimate the center settlement of a slab to be ½ to 1 inch for a uniform loading of up to 150 psf. A vapor barrier should be used beneath the slab. In addition, the use of a thin sand or gravel layer beneath the slab should be considered.

The estimated settlement should occur over most of the loaded area while the edge settlements should be approximately one–half (1/2) of the center settlement and may only occur over a limited range near the perimeter. In view of the magnitude of the estimated settlements and to bridge any undetected soft or loose areas, good rigidity should be assured in the foundation to minimize the effects of differential settlements.

Adequate steel reinforcement should be designed and included within the foundation. If the estimated settlements for shallow footings or slabs are considered prohibitive, deep foundations should be used to support the structure.

We understand post tensioned slabs may be utilized for the project. If post tension slabs are used, the following design parameters are recommended for use in design.

Edge Moisture Variation (Em):
Edge = 5.1 feet
Center = 8.3 feet

Differential Soil Movement (Ym):
Edge = 0.15 inches
Center = 0.71 inches

Fill Materials

Any "soft" soils within excavations should be removed to a depth where stiffer soils are encountered or to a minimum depth of 2 feet. Excavated soils should be replaced with controlled-compacted structural fill. This fill should consist of clean, select fill material free from debris or organic matter and may be a sand soil (SM, SP, or SW) with less than 10% passing the U.S. No. 200 Sieve. Alternatively, a lean clay soil (CL) may be used. The clay fill should have a Liquid Limit of 40 or less and a maximum Plasticity Index (PI) of 20.

Fill Placement and Compaction

Fill may be placed in 10 to 12-inch loose lifts. Minimum compaction criteria of a dry density at least equal to 95% of its maximum, as determined by the Standard Proctor compaction test (ASTM D698A), should be used.

Inspection and Protection of the Bearing Surface

Inspection of the foundation excavations by a qualified geotechnical engineer or technician should be performed prior to concrete placement to ensure that the proper bearing surface is present. The soils that form the bearing stratum are clays which can undergo severe loss of strength when wetted.

DEEP FOUNDATIONS

As an alternative to shallow footings, heavier loaded or settlement sensitive structures may be supported on drilled, cast-in-place, concrete shafts. The following paragraphs provide details regarding drilled shafts.

Allowable Shaft Load Capacities

Analyses have been made to determine the estimated allowable shaft load capacities for drilled, cast-in-place, concrete shafts. Allowable shaft load capacities are provided on Figure 2.

The allowable shaft load capacities presented on Figure 2 provide for a 2-foot discount of soil capacity below the existing ground surface, assume the shafts are vertical, and do not include the weight of the shaft. The provided compression capacities contain an estimated factor of safety of 2 against failure of a single shaft through the soil. The provided tension capacities contain an estimated factor of safety of 3 against failure. The capacities also include a limiting adhesion value based on load tests in geologically similar soils.

The analyses for shaft capacities are based on a soil-shaft relationship only. The structural capacity of the shafts and their connections to transmit these loads should be determined by a structural engineer.

Shaft Installation

Based on the groundwater observations made during the field exploration, casing or slurry displacement techniques do not appear necessary. An experienced drilled shaft contractor should be contacted to determine the proper installation techniques in this area.

Slurry, if used, should be introduced within five feet of starting the shaft, regardless of if casing is used in conjunction with the slurry installation. The contractor performing the slurry displacement method should be qualified and experienced in this method of installing shafts.

The concrete for the shafts should be placed as soon as possible after the excavations are completed. No excavation should be allowed to remain open for more than 1 hour. The concrete should be tremied into place by pumping.

The concrete mix (water to cement ratio) should be proportioned to achieve the necessary design strength while allowing a slump of 6 to 8 inches (or approved mix design) during concrete placement. There have been problems with both casing withdrawal and shaft integrity and capacity when concrete with a slump of less than 6 inches was used. A program of on-site quality control by a qualified geotechnical technician is strongly recommended during shaft installation.

Estimated Settlement of Shaft Foundations

No detailed settlement analyses were made since the design structural loads, shaft layout, etc. are not known. However, settlement of shaft supported footings using the recommended shaft load capacities in single widely spaced rows or in clusters of up to 4 to 6 shafts is estimated to be on the order of 1 inch or less. Shaft spacing should be designed in accordance with the criteria outlined in Figure 3.

Settlement will increase with the size of the shaft cluster and, if larger clusters of closely spaced shafts are needed for support, detailed settlement analyses should be made. These estimates are based on a loading of 100% of the allowable values.

Our estimates do not include the elastic deformation of the shafts, which should be added to the settlement estimates. Elastic deformation of the shafts may be estimated at 67% of the static column strain of a shaft acting as a column. In the event any of our assumptions are not valid, LTI should be contacted to evaluate the potential effects on settlement of shaft foundations.

Drag Load

When fill is placed on the site, the underlying compressible soils consolidate, resulting in surface settlement. As the compressible soils consolidate, "negative skin friction" or downdrag can be imparted on shafts. This can result in a load that is additive to structural loads on the shafts and will increase settlement of the shafts and structure.

Drag load is dependent on the thickness of fill, compressibility of the soils, time-rate of consolidation, and shaft size and length. If no more than 2 feet of fill is placed to raise the building site, the effects of fill and downdrag on shafts will be minimal. LTI should be notified if more than 2 feet of fill is placed to raise the building site.

Group Effect

The effects of shaft grouping on single shaft load capacities are dependent on shaft spacing, shaft lengths, and soil characteristics throughout the shaft length and below the shaft tips. Assuming a minimum center to center spacing of 3 ft., group effect should be unimportant for shaft clusters of up to 6 shafts. Group effects may become important for larger clusters and should be evaluated when actual shaft layouts are known using the criteria provided on Figure 3.

CONSTRUCTION CONSIDERATIONS

Poor site conditions will develop unless good drainage is provided throughout the project duration. Proper site drainage should be maintained prior to, during, and after construction. Providing drainage during the construction process will facilitate construction by reducing the potential for compaction problems. Maintaining the drainage after construction will improve the life of the foundation by avoiding water softening of the foundation soils.

Prior to construction, the site should be stripped of all debris, vegetation, etc., and proof rolled with a heavy wheeled vehicle to detect any "soft" spots. Any soft spots should be undercut at least 2 feet and backfilled with structural fill. At the time of drilling, the site was grass covered. Therefore, 6 to 12 inches of stripping should be anticipated.

The methods, means, and sequence of construction are the responsibility of the contractor. Appropriate measures should be taken by the contractor to assure the integrity and performance of the foundations during and after construction.

CLOSING

LTI is available to answer any questions you may have concerning this report. Should additional analyses be required or requested, additional fees may be necessary.

We appreciate the opportunity to provide this report and look forward to working with you again in the future.

FIGURES

LTI File 4603-BO

LOUISIANA TESTING & INSPECTION, INC.
LAFAYETTE, LA

St. Julien Park Pickleball Courts Broussard, LA	**For City of Broussard C/O Comeaux Engineering Broussard, LA**	**BORING PLAN**
		Figure No. 1

GEOTECHNICAL INVESTIGATION

ST. JULIEN PARK PICKLEBALL CORUTS
ST. NAZAIRE ROAD
BROUSSARD, LA

LOUISIANA TESTING AND INSPECTION PROJECT NO. 4603-BO

ALLOWABLE SHAFT LOAD CAPACITIES

DRILLED, CAST IN PLACE, CONCRETE SHAFTS

SHAFT SIZE	SHAFT TIP EMBEDMENT BELOW GROUND SURFACE IN FEET	ESTIMATED ALLOWABLE SINGLE SHAFT LOAD CAPACITIES IN TONS COMPRESSION FACTOR OF SAFETY = 2 TENSION FACTOR OF SAFETY = 3	
		COMPRESSION	TENSION
12-inch Diameter	15 20	8 10	5 7
14-inch Diameter	15 20	9 12	6 8
16-inch Diameter	15 20	11 14	7 9

Minimum Pile/Shaft Spacing

$SP = 0.05\ L_1 + 0.025\ L_2 + 0.0125\ L_3$

SP (ft.) = Center to center spacing of piles/shafts = (Min. 3.0 ft.)

L_1 = Pile/Shaft penetration in ft. up to 100 ft.

L_2 = Pile/Shaft penetration in ft. from 101 to 200 ft.

L_3 = Pile/Shaft penetration in ft. from 201 to 300 ft.

Allowable Group Capacity*

$$Q_a = \frac{P * L * c}{FSF} + \frac{2.6 * q_u * (1 + 0.2\ {}^{w}/_{b}) * A}{FSB}$$

P = Average perimeter of pile/shaft group (ft.)

L = Length of piles/shafts in group (ft.)

c = Average (weighted) shear strength (½ q_u) of soil throughout pile/shaft length (lbs./sq. ft.)

q_u = Unconfined compressive strength of soils below pile tips (lbs./sq.ft.)

w = Width of pile/shaft group at tip (ft.)

b = Length of pile/shaft group at tip (ft.)

A = Area of pile/shaft group at tip (sq. ft.)

FSF = Factor of safety for friction area = 2

FSB = Factor of safety for tip area = 3

*In no case should the cumulative single pile/shaft load capacity of the group be exceeded.

APPENDIX

BORING LOGS

BORING NO. B-1

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Coord: 30° 8'19.32"N, 91°55'54.98"W Description of Stratum
0		2.00 (PP)	0.76	30.7	76	30	11			Medium Stiff dark brown and gray SILTY CLAY (CL) with organic matter
		3.25 (PP)	0.67	25.2	86	35	20			
5		4.00 (PP)	0.47	26.3	84					Soft dark brown SILTY CLAY (CL)
		3.25 (PP)	1.16	24.3	89	32	16			Stiff brown and gray SILTY CLAY (CL)
		3.00 (PP)	1.13	26.4	86	37	21			
10										
		1.50 (PP)	1.02	27.8	86	32	12			
15										
										Very Soft brown and gray SILTY CLAY (CL)
		1.00 (PP)	0.15	32.6	79					
20										
		1.00 (PP)	0.51	30.0	83	30	10			Medium Stiff brown and gray SILTY CLAY (CL)
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-2

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Coord: 30° 8'19.21"N, 91°55'53.65"W Description of Stratum
0		3.50 (PP)	1.41	19.9	87	29	9			Stiff brown and gray SILTY CLAY (CL) with organic matter (Dry)
		2.50 (PP)	0.64	26.3	78					Medium Stiff brown and gray SILTY CLAY (CL)
5		3.25 (PP)	0.92	25.9	76					
		2.50 (PP)	0.58	25.5	77					
		1.75 (PP)	0.88	27.3	82					
10										
		1.50 (PP)	0.70	31.3	79	37	18			
15										
										Stiff tan and gray SILTY CLAY (CL)
		2.25 (PP)	1.24	20.7	95	29	12			
20										
		2.25 (PP)	1.79	21.2	97	42	26			
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-3

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Coord: 30° 8'18.92"N, 91°55'54.55"W Description of Stratum
0		4.50+ (PP)	3.18	22.6	98	37	16			Very Stiff brown and gray SILTY CLAY (CL) with organic matter (Dry)
		4.50+ (PP)		17.6		34	17			
5		4.50+ (PP)	1.57	15.9	77	31	12			Medium Stiff to Stiff brown and gray SILTY CLAY (CL) (Dry)
		2.75 (PP)	0.72	18.7	72					
		2.75 (PP)	1.17	26.1	82					Medium Stiff to Stiff tan and gray SILTY CLAY (CL)
10										
		1.75 (PP)	0.57	25.8	86	31	13			
15										
										Very Stiff tan and gray SILTY CLAY (CL)
		3.25 (PP)	3.34	20.0	98	47	28			
20										
		4.50+ (PP)	3.19	18.6	100	46	29			
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-4

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Coord: 30° 8'18.60"N, 91°55'54.95"W Description of Stratum
0		4.50+ (PP)	2.11	21.2	91	27	11			Very Stiff dark brown and gray SILTY CLAY (CL) with organic matter
		3.00 (PP)	0.84	25.9	83					Medium Stiff dark brown and gray SILTY CLAY (CL)
5		3.00 (PP)	0.36	24.9	88					Soft dark brown and gray SILTY CLAY (CL)
		2.25 (PP)	0.93	24.1	89					Medium Stiff to Stiff brown and gray SILTY CLAY (CL)
10		1.25 (PP)	1.17	27.5	85					
		1.25 (PP)	0.44	27.2	86					Very Soft to Soft brown and gray SILTY CLAY (CL)
15										
		1.25 (PP)	0.21	26.9	85					
20										
		1.25 (PP)	0.26	29.3	79					
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-5

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Coord: 30° 8'18.31"N, 91°55'53.69"W

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Description of Stratum
0		3.00 (PP)	0.55	27.6	73					Medium Stiff brown and gray SILTY CLAY (CL) with organic matter
		3.00 (PP)	0.91	26.3	83					
5		1.25 (PP)	0.79	25.2	78					
		1.75 (PP)		25.6						Soft brown and gray SILTY CLAY (CL)
		1.75 (PP)	0.37	28.9	73					
10										
		1.50 (PP)	0.67	31.9	79					Medium Stiff brown and gray SILTY CLAY (CL)
15										
		3.00 (PP)	2.11	20.4	95					Very Stiff tan and gray SILTY CLAY (CL)
20										
		3.75 (PP)	2.02	23.5	92	41	22			
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-6

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Coord: 30° 8'17.48"N, 91°55'53.51"W

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Description of Stratum
0		4.50+ (PP)	4.35	17.3	101	39	21			Hard brown and gray SILTY CLAY (CL) with organic matter (Dry)
		4.50+ (PP)	1.01	18.8	93	34	17			Stiff brown and gray SILTY CLAY (CL) (Dry)
5		4.50+ (PP)	1.77	23.1	84					
		2.25 (PP)	0.72	22.3	77					Medium Stiff brown and gray SILTY CLAY (CL)
		3.00 (PP)		22.7						
10										
		2.00 (PP)	0.87	28.3	81					
15										
		3.50 (PP)	2.70	21.1	97					Very Stiff tan and gray SILTY CLAY (CL)
20										
		3.25 (PP)		21.9						
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-7

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Coord: 30° 8'17.45"N, 91°55'52.61"W

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Description of Stratum
0		4.25 (PP)	0.82	23.4	86	43	21			Medium Stiff brown and gray SILTY CLAY (CL) with organic matter
		4.50+ (PP)	3.66	18.1	97	42	19			Very Stiff brown and gray SILTY CLAY (CL)
5		2.50 (PP)		23.1						
		2.50 (PP)		22.3						
		2.00 (PP)	0.92	24.7	84					Medium Stiff to Stiff brown and gray SILTY CLAY (CL)
10										
		2.25 (PP)	1.00	28.6	80	35	14			
15										
		2.25 (PP)	2.48	21.0	98	40	23			Very Stiff tan and gray SILTY CLAY (CL)
20										
		1.50 (PP)	0.75	23.6	91	27	11			Medium Stiff tan and gray SILTY CLAY (CL)
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

BORING NO. B-8

Project: St. Julien Park Pickleball Courts
St. Nazaire Road

LTI File No.: 4603-BO
Date Start: 2/3/2025

Location: Broussard, LA
Client: City of Broussard C/O Comeaux Engineering
Broussard, LA

Depth (Feet)	Sample	(Field Test) PP/ SPT	Comp. Strength (tsf)	Water Content (%)	Dry Density (pcf)	LL (%)	PI (%)	% Pass No. 200 Sieve	TYPE	Coord: 30° 8'17.20"N, 91°55'51.71"W Description of Stratum
0		4.50+ (PP)	2.04	25.4	92					Very Stiff brown and gray SILTY CLAY (CL) with organic matter (Dry)
		4.00 (PP)		17.5						
5		2.00 (PP)		16.9						
		3.00 (PP)	0.79	18.6	69					Medium Stiff brown and gray SILTY CLAY (CL)
		2.50 (PP)	1.23	28.2	82	37	15			Stiff brown and gray SILTY CLAY (CL)
10										
		2.50 (PP)	0.50	75.9	90					Medium Stiff tan and gray SILTY CLAY (CL) with organic matter
15										
		3.75 (PP)	1.06	22.2	91					Stiff to Very Stiff tan and gray SILTY CLAY (CL)
20										
		3.75 (PP)	3.05	20.0	99					
25										Boring complete at 24 feet below ground surface.
30										
35										
40										

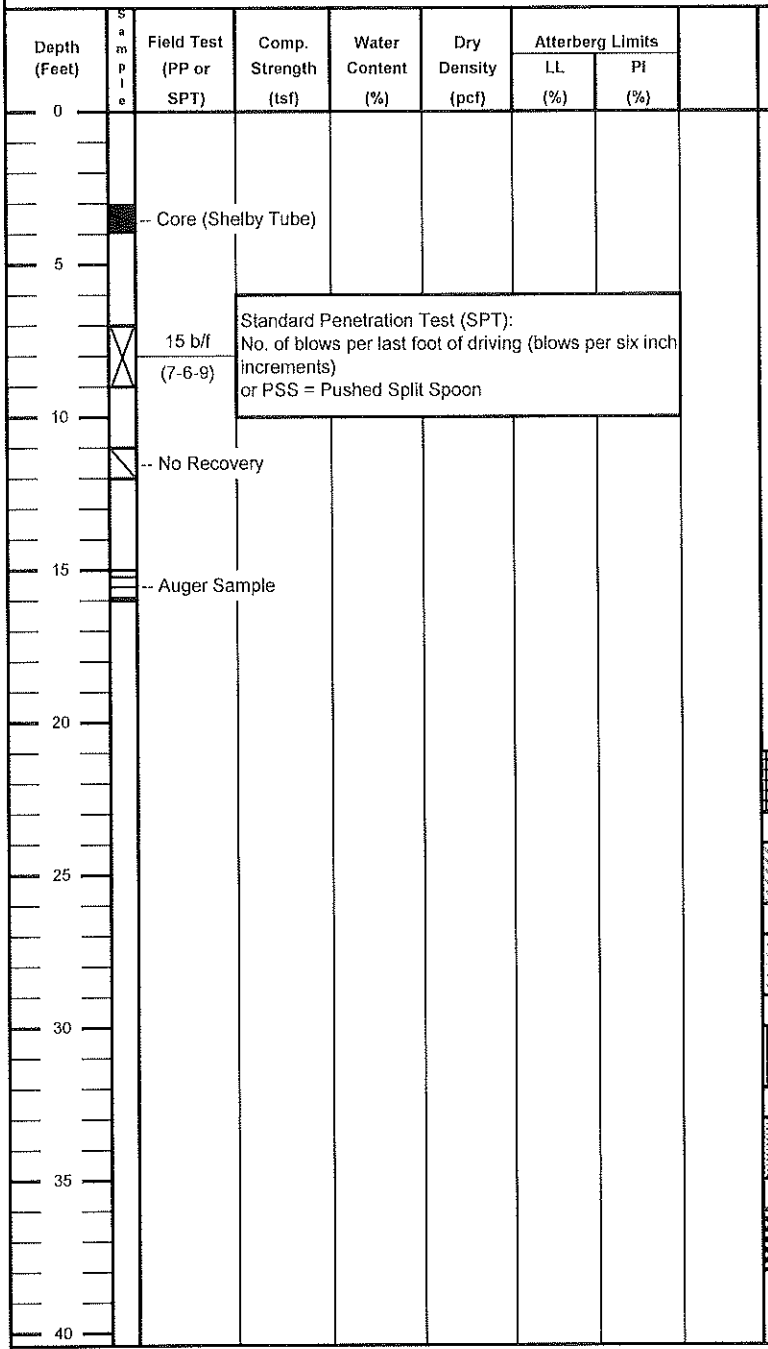
Sample Legend:
- Core (Shelby Tube)
- Standard Penetration (SPT)
- No Recovery
- Auger Sample

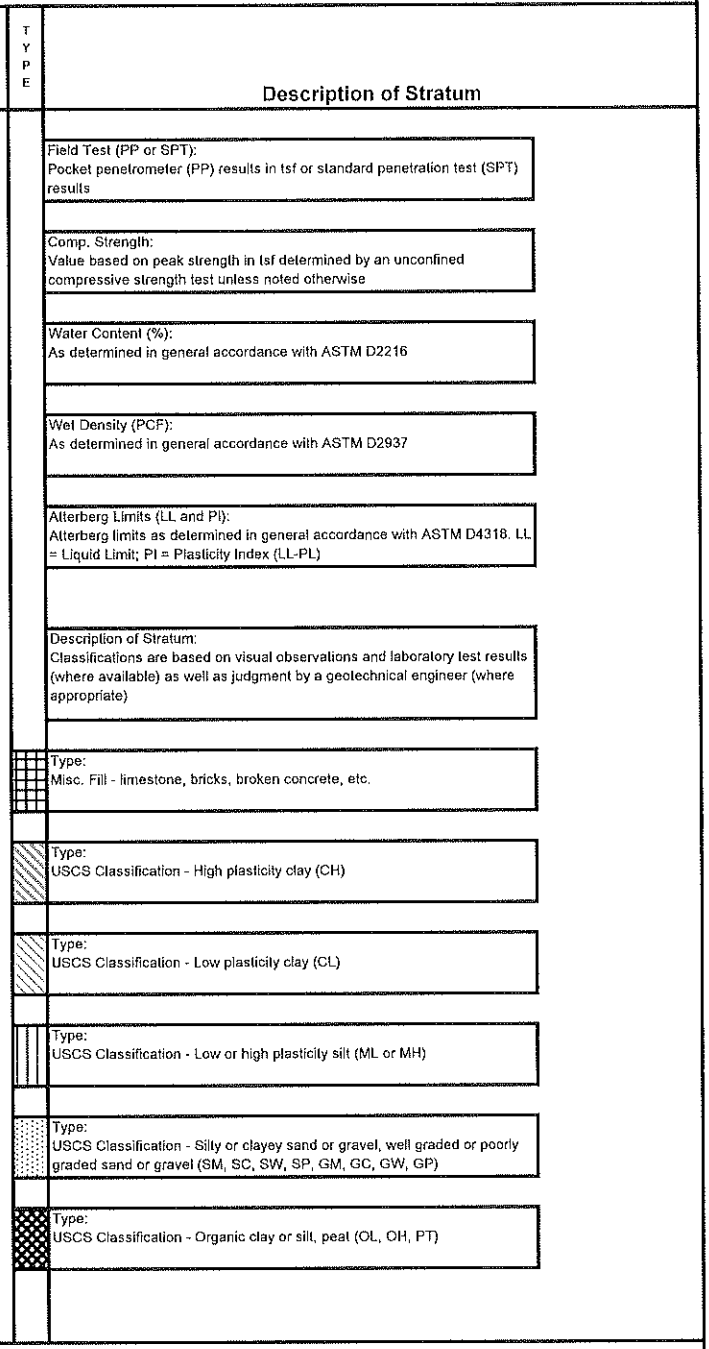
Comments/Notes:
- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion
- Free Water = Not encountered

LOUISIANA TESTING AND INSPECTION, INC.

LAFAYETTE, LA

SOIL BORING LOG - DESCRIPTION OF TERMS AND SYMBOLS





Sample Legend:

- Core (Shelby Tube)
- Standard Penetration Test (SPT)
- No Recovery
- Auger Sample

Comments/Notes:
General notes or comments regarding boring and data

LOUISIANA TESTING AND INSPECTION, INC.

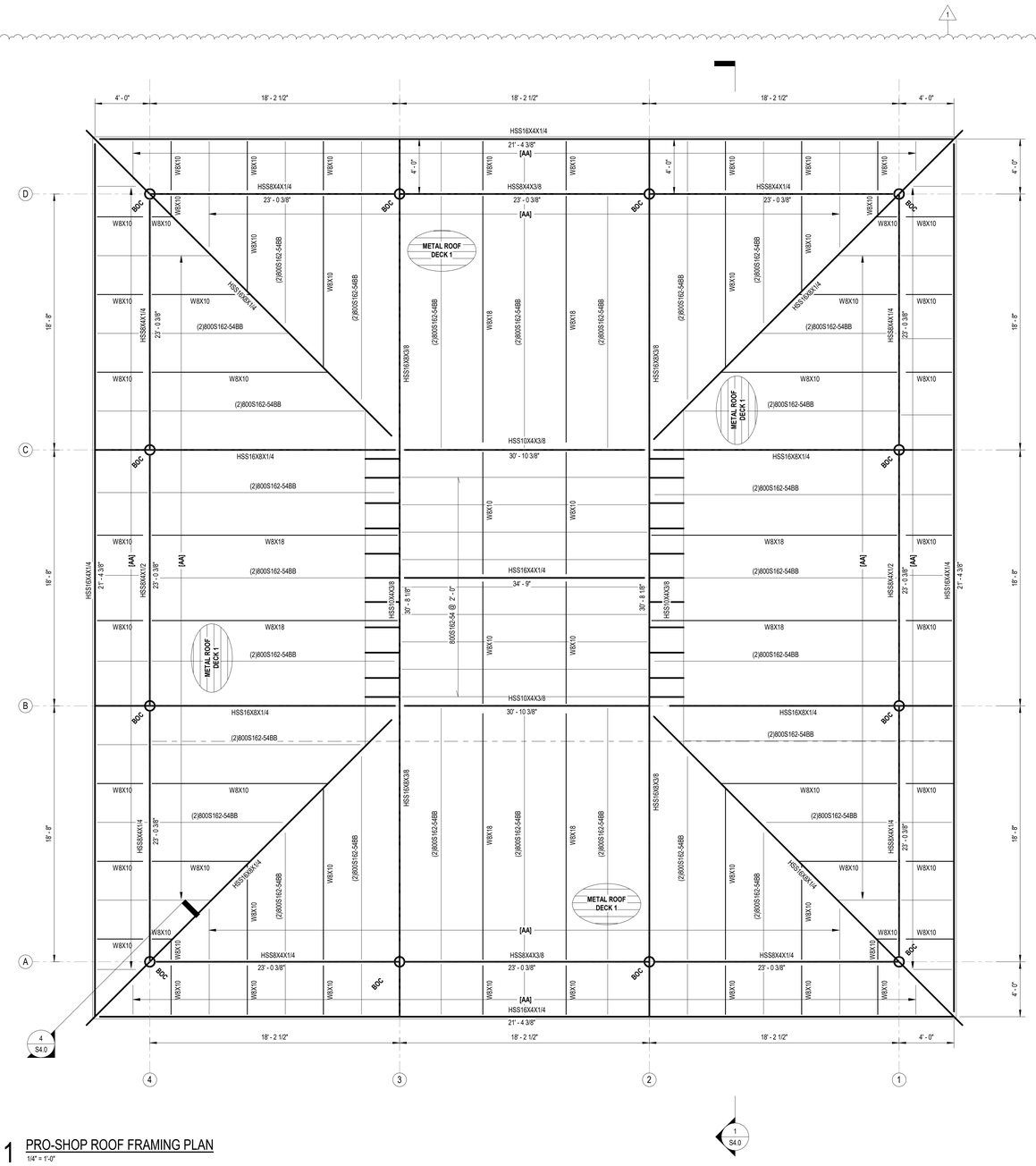
LAFAYETTE, LA

Louisiana Testing & Inspection, Inc.

Lafayette, Baton Rouge, Houma

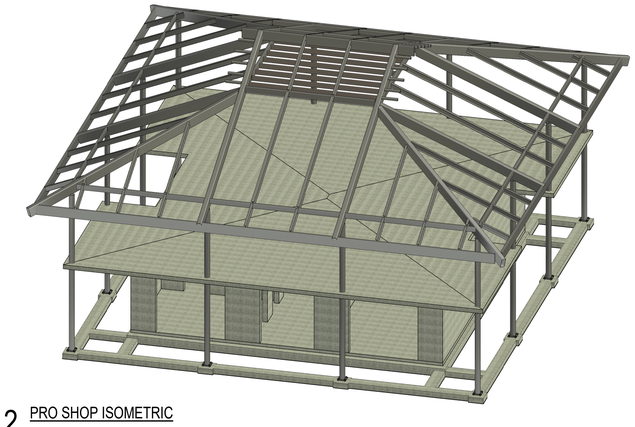
P.O. Box 2934
Lafayette, LA 70502

E-Mail: LouisianaTesting@GMAIL.com
PN: 337.235.9411
FN: 337.232.9362



1 PRO-SHOP ROOF FRAMING PLAN

1/4" = 1'-0"



2 PRO SHOP ISOMETRIC

GENERAL FRAMING NOTES:

ALL JOISTS SHALL BE EQUALLY SPACED BETWEEN SUPPORTS AT 6'-6" O.C. MAXIMUM.

SUPPLY SUPPLEMENTAL FRAMING AS REQUIRED FOR ROOF PENETRATIONS.

GENERAL CONTRACTOR SHALL COORDINATE THE STRUCTURAL FRAMING SYSTEM WITH ARCHITECTURAL AND MEP DRAWINGS.

REFER TO STRUCTURAL SPECIFICATIONS, NOTES, DETAILS, AND SCHEDULES FOR ALL INFORMATION NOT SHOWN.

1

NOTE (AA):
FURNISH 800S162-54 JOISTS (UNLESS NOTED AS DOUBLE JOIST ON PLANS) CENTERED BETWEEN WIDE FLANGE BEAMS TO SUPPORT SOFFIT FRAMING
FASTEN EA END WITH RCA225/54 CONNECTORS (6) #12s EA LEG
FRAME OUT OPENINGS AS REQUIRED FOR FIXTURES RE: ARCHITECTURAL

Trahan
ARCHITECTURE + PLANNING,LLC
819 Saint John St.
Lafayette, La. 70501
337-504-758
tbissell@trahanarchitecture.co

A NEW RACKET CENTER / OBSERVATION DECK FOR

ST JULIEN RECREATIONAL FACILITY

701 ST. NAZAIRE ROAD, BROUSSARD, LA 70518

SOUTHEAST ENGINEERS & LAND SURVEYORS
LOUISIANA | MISSISSIPPI | TEXAS
SCOTT, LOUISIANA 70583

STATE OF LOUISIANA
AUSTIN K. TONEY
License No. 43720
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

DRAWN BY: AKT

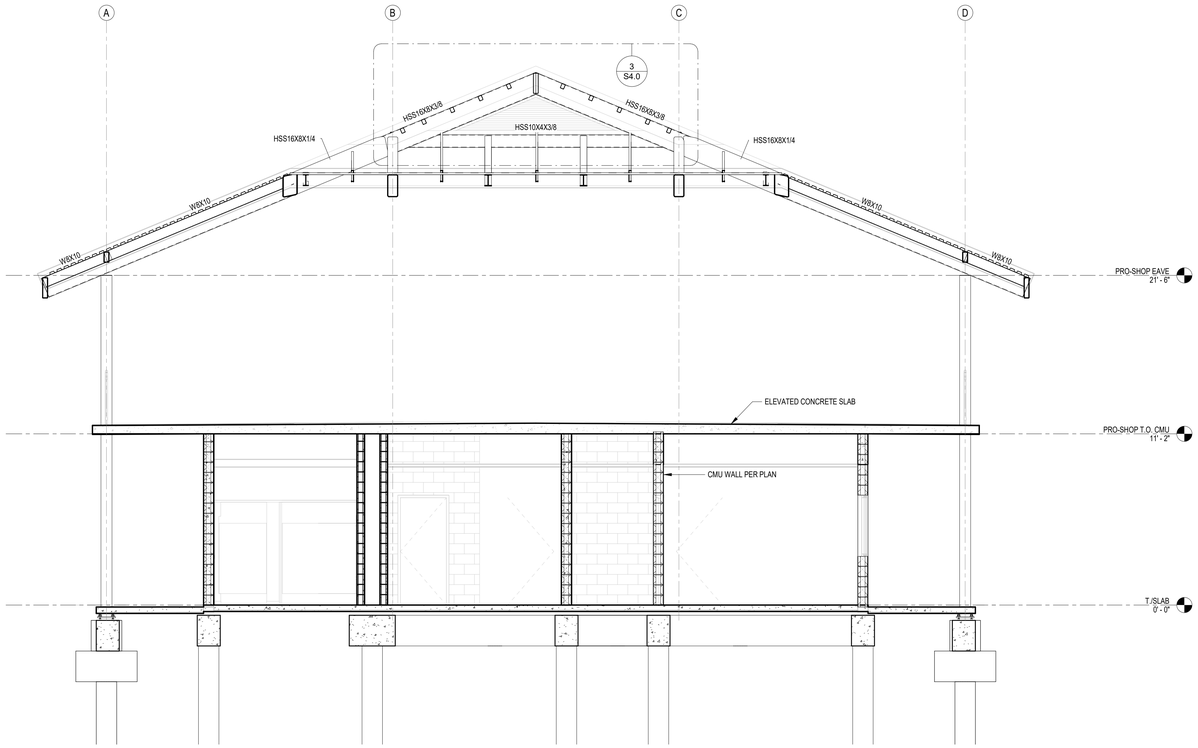
DATE: January 5, 2026

REVISION NO./ DATE: REV-1 2/9/2026

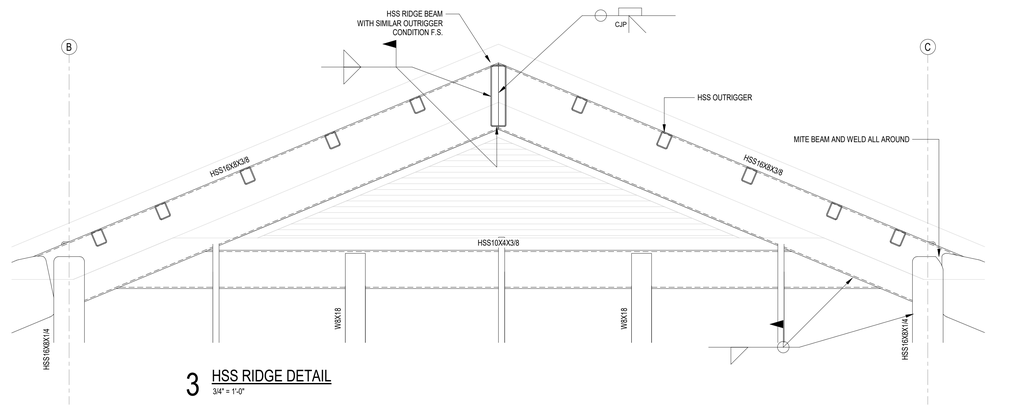
JOB NO.: SE-24-1258

SHEET: S2.2

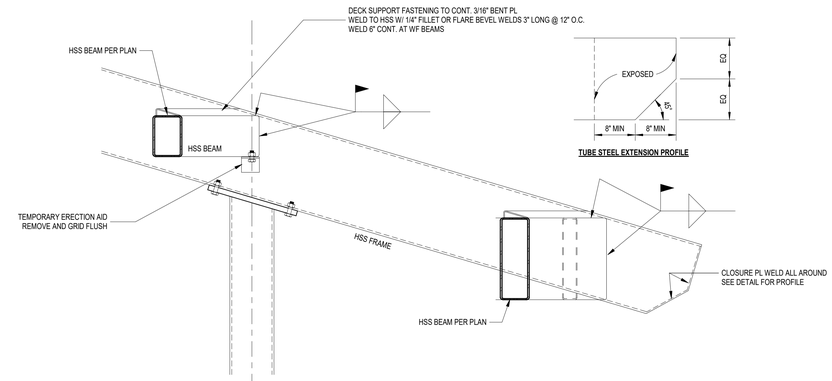
OF SHEETS



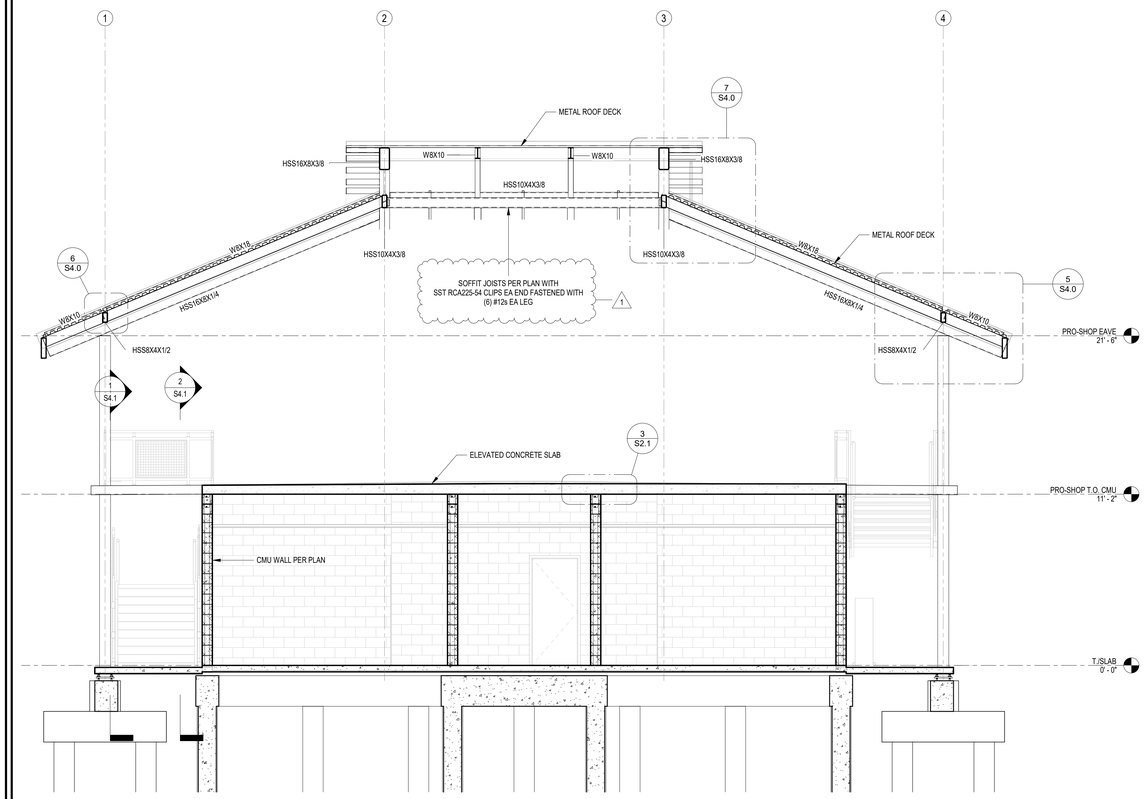
1 PRO SHOP BUILDING SECTION
1/4" = 1'-0"



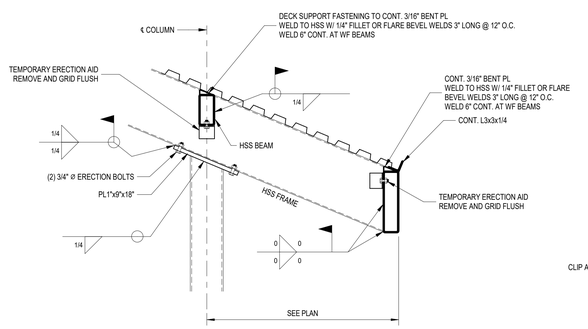
3 HSS RIDGE DETAIL
3/4" = 1'-0"



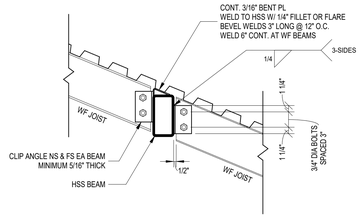
4 HSS ROOF BEAM OVER CORNER COLUMN DETAIL
1" = 1'-0"



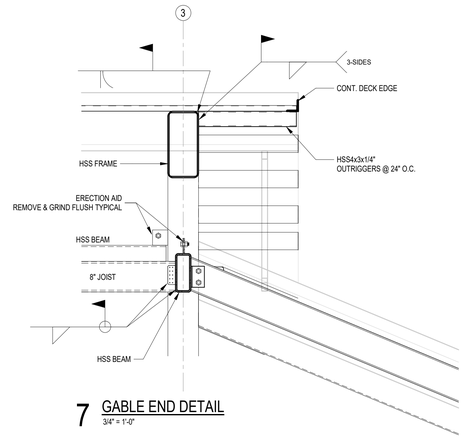
2 PRO SHOP BUILDING SECTION
1/4" = 1'-0"



5 HSS ROOF BEAM OVER COLUMN DETAIL
3/4" = 1'-0"



6 SLOPED WF JOIST TO HSS BEAM DETAIL
1" = 1'-0"



7 GABLE END DETAIL
3/4" = 1'-0"

Trahan
ARCHITECTURE + PLANNING,LLC
819 Saint John St.
Lafayette, La. 70501
337-504-758
tlussell@trahanarchitecture.co

A NEW RACKET CENTER / OBSERVATION DECK FOR

ST JULIEN RECREATIONAL FACILITY

701 ST. NAZAIRE ROAD, BROUSSARD, LA 70518

SOUTHEAST ENGINEERS & LAND SURVEYORS
LAFAYETTE, LOUISIANA 70503
SCOTT, LOUISIANA 70583

STATE OF LOUISIANA
AUSTIN K. TONEY
License No. 43720
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

DRAWN BY: AKT
DATE: January 5, 2026
REVISION NO./ DATE: REV-1 2/9/2026
JOB NO.: SE-24-1258
SHEET: S4.0
OF SHEETS